Enhance the Performance of Raft Foundation with Piles

Tapabrata Roy, Shyamalima Mukherjee, Shramana Ghosh, Amit Biswas, Shubhajit Saha

Abstract---- A FE – BE coupling technique is used in the present study. The raft is idealised as a thick plate freely resting on soil medium, which is idealised as a semi-infinite, isotropic and homogeneous elastic half -space. Pile is idealised as a spring of equivalent stiffness, obtained from simplified approach based on elastic theory assuming it to be a shaft in the half-space. Boundary element method is employed to determine the soil stiffness matrix by inverting the soil flexibility matrix, using Mindlin's solution for a point load in half-space as a fundamental solution. Finite element method is employed to determine the raft stiffness matrix based on Mindlin's plate bending theory, which allows transverse shear deformation. Combined stiffness matrix of the soil-pile-raft system is obtained by summing up the stiffness of the soil-raft system and the stiffness contribution of the piles at selected locations. A computer programme is developed, based on the procedure describe above, in which discretisation is automatic and requires very nominal data input. Effect of pile parameter on settlement of a piled raft system subjected to uniformly distributed load is studied for different number and configuration of piles to demonstrate the efficacy of piled raft system in reducing settlement.

Index Terms--- Boundary element method, Finite element method, Coupling, Elastic half-space, Elastic settlement, Piled raft foundation, Raft foundation.

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1 INTRODUCTION

N designing piles as foundation support for structures, it is

common trend to include a pile cap for joining of the pile heads. The pile cap is designed for structural capacity only. But the pile cap has additional influence on the foundation system besides joining of pile heads and simple load transferring. The roll of the pile cap becomes significant if the cap is in direct contact with the foundation soil. A Piled Raft foundation acts as a composite construction consisting of three load bearing elements: piles, raft and subsoil. According to its stiffness, the raft distributes the total load of structures as contact pressure and over the piles in the ground. The piled raft concept needs evaluation of a number of factors in order to come up with analysis or design models that simulate the actual site conditions. The use of piled raft concept has lead to reduction of total as well as differential settlements. In many cases using raft foundation only induces excessive settlements which are not acceptable due to serviceability requirements. Placing of piles in systematic manner under the raft reduces such settlements to acceptable values. In addition to settlements, the bearing capacity of the whole system of foundation also improves. The conventional design methods used for pile groups lead to a higher number of piles under the raft. With the concept of piled raft, this number can be

 Subhajit Saha, pursuing B-Tech Degree in Civil Engineering from Camellia Institute of Technology, India. Contact No.: 9734945203 reduced. It is seen that piled rafts to be economical solution in foundation design for soil condition where such design is applicable.

2 OBJECTIVE OF PRESENT STUDY

The objective of the present study is to predict the performance of raft foundation using Pile. A coupled Finite Element - Boundary Element approach has been used to determine the stiffness matrix of the combined foundation system. The Raft foundation has been considered as a thick plate. Boundary Element Method and Finite Element Method have been employed to find out the soil stiffness matrix and plate stiffness matrix respectively. The soil has been considered as an elastic continuum. The pile stiffness has been represented by the spring of equivalent stiffness. The pile stiffness has been determined by using elastic theory by determining the settlement of pile and the pile has been considered as a shaft in the half-space. The half - space response is based on the solution given by Mindlin (1936) for a point load in half space, which allows taking into account the effect of embedment of the plate. Combined stiffness of the soil-pile-raft system has been obtained by summing up the stiffness of the soil raft system and the stiffness contribution at the pile locations.

Computer programmes have been developed in FORTRAN language, in which discretisation is automatic and requires very nominal data input. The result has been verified with available solution in literature. A practical problem has been solved indicating the predicted settlement for different pile configuration to justify the efficacy of such study.

Tapabrata Roy, Assistant Professor, Civil Engineering Department, Camellia Institute of Technology. India. Contact No.: 9874329041

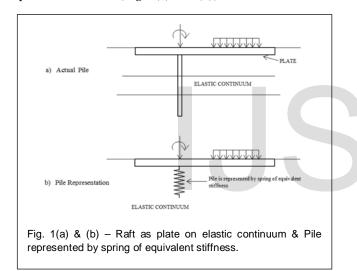
Shyamalima Mukherjee, pursuing B-Tech Degree in Civil Engineering from Camellia Institute of Technology, India. Contact No.: 9748802412

Shramana Ghosh, pursuing B-Tech Degree in Civil Engineering from Camellia Institute of Technology, India. Contact No.: 9038959153

Amit Biswas, pursuing B-Tech Degree in Civil Engineering from Camellia Instituter (1990) INSER © 2016
Amit Biswas, pursuing B-Tech Degree in Civil Engineering from Camellia Instituter (1990) Instituter

3 FORMULATION OF HYPOTHESIS AND CONCEPTUAL MODEL

The half-space response is based on the solution given by (Mindlin 1936) for a point load in half space, which allows taking into account the effect of embedment of the plate. The plate and the half-space are two separate models in unilateral and frictionless contact at the interface. Plate-half-space interface is discretised into two dimensional isoparametric quadrilateral quadratic elements and the plate is discretised into eight nodded isoparametric plate bending finite elements based on Mindlin's plate bending theory, which allows for transverse shear deformation. The stiffness matrix obtained from the boundary element method (by inverting the flexibility matrix) is coupled with the plate stiffness matrix obtained from the finite element method after transformation to get the stiffness matrix of the plate-half-space system [Mandal and Ghosh (1999)]. Pile is idealised as a spring of equivalent stiffness [Fig. 1(a) and (b)].



The stiffness of the pile is obtained from simplified approach based on elastic theory [Poulos and Davis (1980)]. The pile is considered to be a shaft in the half-space subjected to axial load of P at the ground surface (Fig. 2). The sides of the pile are assumed to be rough. The soil is considered to be an ideal homogeneous isotropic elastic half-space, having elastic parameters Es and v_{s} .

4 METHODOLOGY

Combined stiffness of the pile raft system $[K_{\rm pr}]$ has been obtained by summing up the stiffness of the soil- raft system and the stiffness contribution at the pile locations.

$$[K_{pr}] = [K_{plate}] + [K_{soil}] + [K_{pile}]$$

Where, $[K_{plate}]$ is the stiffness of the plate, $[K_{soil}]$ is the stiffness of the soil and $[K_{pile}]$ is the stiffness of the piles. The force displacement equation of the piled raft system can be written as

P d Soil Young's Modulus (Es) & Poisson's ratio (v_s) Rigid Stratum Fig. 2 - Pile in elastic continuum

 ${u_i}_c$ is the generalised displacement at the nodes and ${P}_c$ is the applied external load at the nodes. By solving the above equation displacement parameters can be obtained.

A computer code has been developed in FORTRAN language, based on the on theoretical study. It can determine the elastic settlement of raft foundation placed on elastic medium without and with piles at any location of the raft. The influence of introduction of pile on settlement can be studied and recommendations can be made for optimum use of piles and thickness of raft for settlement control.

5 DATA ANALYSIS

Analysis of a flexible raft foundation subjected to combination of uniformly distributed load & concentrated load is carried out with and without pile foundation. The effect of length of the pile & pile dimensions on settlement is carried out to justify the efficacy of using piled raft foundation for settlement control and to minimize the thickness of raft foundation.

To validate the theoretical study and the computer program a comparative study is carried out with a problem as given below "Mendonca and Paiva (2003)" is considered.

A piled raft with dimensions 30.0 m × 15.0 m × 0.25 m, Young's modulus $E = 2.0 \times 10^7$ KN/m² and Poisson's ratio (ν) = 0.2, is supported on soil and on nine piles (Fig. 3). In the raft stiffened by the piles, the set of piles consists of groups P₁, P₂, P₃ and P₄. The mechanical properties of the soil are Young's modulus (E_s) = 2.0 × 10⁵ KN/m² and Poisson's ratio (ν_s) = 0.5. All the piles are of length 15 m and diameter (D_p) = 0.3 m and when they are considered to be flexible, their Young's modulus (E_p) is = 2.0 × 10⁷ KN/m². A uniform load g = 30.0 KN/m² is applied across the whole surface of the plate.

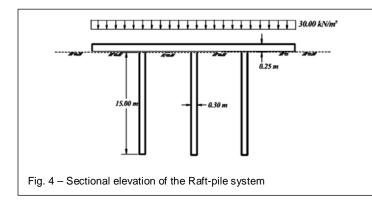
For study raft will be considered to be supported by nine piles as given in the Fig. 3 & 4.

 $\{u_i\}_c = [K_{pr}]^{-1}\{P\}_c$

(2)

(1)

The raft plate is discretised into 72 elements (12 division in the longer direction and 6 divisions in shorter direction) and the discretisation scheme for the above problem is in Fig. 5.



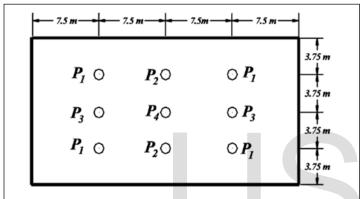
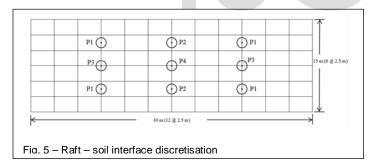
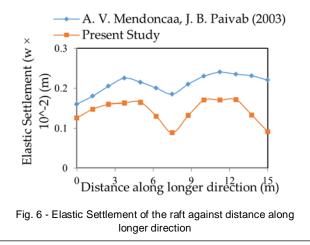


Fig. 3 - Plan of the arrangement of piles below the raft foundation



The result of the present analysis is compared with the solution obtained by Mendonca and Paiva (2003) and has been plotted in Fig. 6.

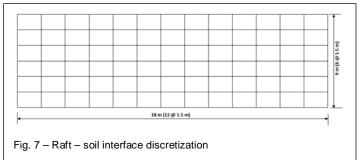


The ranges of settlement predicted are in close proximity with the published result. The predicted settlements for the project study are on the lower side. This may be attributed to the modeling of the raft as a thick plate which considered shear deformation into account.

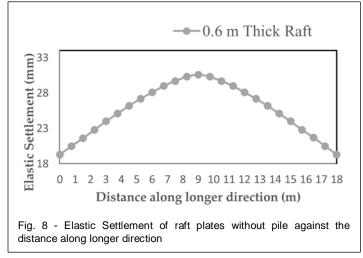
A rectangular 0.6 m thick raft plate of dimension 18 m × 9 m is placed on a sandy soil deposit having soil modulus (E_s) = 2 × 10⁴ KN/m² and Poisson's ratio (μ_s) = 0.3. Raft modulus (E_p) = 2.5×10⁷ KN/m² and Poisson's ratio (μ_r) = 0.2. Loading condition is combined; uniformly distributed load = 40 KN/m² and concentrated load = 1520 KN.

Now the above problem on a rectangular raft foundation has been analysed without pile.

The raft plate is discretised into 72 elements (12 division in the longer direction and 6 divisions in shorter direction) and the discretisation scheme for the above problem is in Fig. 7.

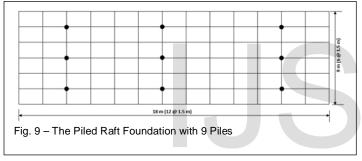


The evaluated elastic settlement of the above mentioned different raft plates without pile against distance along longer direction has been plotted in Fig. 8.

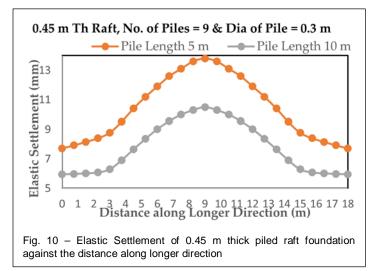


Now the above mentioned problem on a rectangular raft foundation has been analysed with piles.

The arrangement of piles is shown below (Fig. 9) and considering diameter of pile 0.3 m and length of pile 5 m & 10 m and thickness of raft foundation 0.45 m.



Elastic Settlement of the piled raft foundation with 9 piles considering 0.45 m thick raft and diameter of pile 0.3 m, has been determined with different length of piles. The calculated result has been plotted in Fig. 10.



It can be observed that introduction of pile in a raft foundation minimize the thickness of raft foundation and also reduces the maximum settlement quite considerably.

Optimum no. of piles, different length of piles and configuration can be determined depending on the requirement of settlement consideration for a particular problem. Suitable selection of raft thickness and pile configuration can be chosen for optimum design of the piled raft foundation system.

6 CONCLUSION & DISCUSSION

The primary objective of the study is to develop an approach for optimization of piled raft foundation system. The present study has demonstrated that piled raft foundation can be used to reduce elastic settlement and also minimize the thickness of raft. It is observed that use of piles leads to reduction of total as well as differential settlement. The effect of pile dimensions and length of pile on the performance of a piled raft foundation has been studied.

The conclusive remarks can be summarized as below:

[1] A couple BEM – FEM formulation for the analysis of piled raft has been presented in which all the interactions between the plate, the pile and the soil are considered.

[2] The developed computer programme shows good convergence with lesser number of elements. This highlights the suitability of the coupled approach.

[3] Both the stiffness of the raft and piles has a major role in determining the elastic settlement of piled raft foundation.

[4] It is observed that introduction of piles in the system reduces settlement considerably. Hence piled raft can be used to minimize the elastic settlement as required.

[5] For same configuration of piles elastic settlement reduces with increment in length of pile.

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